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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

ROY COOPER
GOVERNOR

JAMES H. TROGDON, III
SECRETARY

DATE: September 14, 2017

TO: Karen E. Collette, P.E.

Division Engineer

FROM: Christopher Kreider, P.E.

Eastern Regional Geotechnical Manager 9/15/2017

STATE PROJECT: 40238.1.4 (U-4902D)

F.A. NUMBER.: NHS-0017(76) COUNTY: New Hanover

DESCRIPTION: US 17 Business (Market Street) from Lendire Road to Marsh

Oaks Drive

SUBJECT: Geotechnical Report Addendum – Rock Plating

Recommendations

Per Division's request, the Geotechnical Engineering Unit has reviewed the roadway plans and cross-sections for the proposed rock plated slopes listed below and provides the following recommendations.

| <u>Line</u> | $\underline{\text{Station}(\pm)}$ | Location |
|-------------|-----------------------------------|----------|
| -L- | $270+75 \pm to \ 278+75 \pm$ | RT |

Use the Standard Rock Plating detail No. 1 of the Standard Drawing No. 1802.01 as shown on the Roadway plans. The estimated quantity for rock plating is 1600 square yards. Rock Plating is outlined in the Section 275 of the 2012 Standard Specifications. The following note should be added to the Roadway Plans:

"USE ROCK PLATING DETAIL NO. 1 AT -L- STA 270+75± TO -L- STA 278+75±, RIGHT. EXTEND ROCK PLATING LIMITS TO 2.75:1 (H:V) SLOPES. SEE STANDARD ROCK PLATING DETAIL."

Telephone: (919) 662-4710

Customer Service: 1-877-368-4968

Website: www.ncdot.gov

Summary of Rock Plating Table

| Line | Beginning Slope | Approx. Station | Ending Slope | Approx. Station | Location LT/RT | Rock Plating Detail No. 1/2/3/4 | Riprap Class 1/2/B | SY |
|------|--------------------|--------------------|-----------------|--------------------|-------------------|---------------------------------------|--------------------------|------|
| -L- | 2.75:1 | 270+75 ± | 2.75:1 | 278+75 ± | RT | 1 | * | 1600 |

^{*} Use Riprap Class B, 1 or 2.

If you have any questions concerning this memorandum, please contact Majid Khazaei, P.E. or Chris Kreider, P.E. at 919-662-4710.

Prepared by,

SEAL
036278

9/15/2017

Majid Khazaei, P.E.

Geotechnical Design Engineer

CAK/MK

SEE SHEET 3 FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

CONTENTS

REFERENCE

| <u>LINE</u> | <u>STATION</u> | <u>PLAN</u> | PROFILE |
|-------------|------------------|-------------|----------------|
| L | 202+50 TO 288+00 | 4-10 | 11-17 |
| Y28A | 10+00 TO 11+25 | 4 | 18 |
| Y29 | 13+00 TO 17+10 | 6 | 18 |
| Y30 | 10+00 TO 11+75 | 6 | 18 |
| Y3I | 11+25 TO 12+50 | 7 | 18 |
| Y32 | 11+50 TO 13+02 | 7 | 18 |
| Y33 | 11+65 TO 12+50 | 8 | 18 |

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

ROADWAY SUBSURFACE INVESTIGATION

COUNTY NEW HANOVER

PROJECT DESCRIPTION US 17 BUSINESS (MARKET ST.) FROM LENDIRE ROAD TO MARSH OAKS DRIVE

INVENTORY

STATE PROJECT REFERENCE NO. 18 U-4902D

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1991 707-680. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS MEDICATED DESCRIPTIONS AND ASSECTIONS OF THE INVESTIGATION. THE STATEM LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS MEDICATED DESCRIPTIONS AND ASSECTIONS AND ASSECTIONS OF THE INVESTIGATION. THE ACCORDING TO CLIMATIC CONDITIONS MEDICATED DESCRIPTIONS AND ASSECTIONS AND ASSECTIONS OF THE ACCORDING TO CLIMATIC CONDITIONS MEDICATED DESCRIPTIONS AND ASSECTIONS AND ASSECTIONS OF THE ACCORDING TO CLIMATIC CONDITIONS MEDICATED DESCRIPTIONS AND ASSECTIONS AND ASSECTIONS OF THE ACCORDING TO CLIMATIC CONDITIONS MEDICATED DESCRIPTIONS AND ASSECTIONS AND ASSECTIONS OF THE ACCORDING TO CLIMATIC CONDITIONS MEDICATED DESCRIPTIONS ASSECTIONS OF THE ACCORDING TO CLIMATIC CONDITIONS MEDICATED ASSECTIONS. INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT, THE DEPARTMENT DOES NOT WARRANT OR CUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPHION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- IES:
 THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT
 OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS
 OR CONTRACT FOR THE PROJECT.
 BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS
 FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE
 CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

COREY FUTRAL T. TRIANTIS W.J. MILLER, CWC MICHAEL D. MASON

PERSONNEL LINDSAY PUGH

INVESTIGATED BY __CATLIN

DRAWN BY __DAVID MCPHERSON CHECKED BY STEVEN V. HUDSON

SUBMITTED BY STEVEN V. HUDSON

DATE __MAY 2016

steve hudson 5/31/2016 SZEFD88181E445F... SIGNATURE

> **DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED**

PROJECT REFERENCE NO. SHEET NO.

U-4902D

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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

| COLL DECCRIPTION | CDADATION | DOCK DECEDIATION | TEDMO AND DEFINITIONS |
|--|---|---|---|
| SOIL DESCRIPTION SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN | GRADATION WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. | ROCK DESCRIPTION HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED | TERMS AND DEFINITIONS ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. |
| BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION | UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. | ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 | AQUIFER - A WATER BEARING FORMATION OR STRATA. |
| IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH | GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES. ANGULARITY OF GRAINS | BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. | ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. |
| AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, | THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: | ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS: | ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING |
| VERY STIFF,GRAY,SILTY CLAY,WOIST WITH INTERBEDDED FINE SAND LAYERS,HIGHLY PLASTIC,A-7-6 SOIL LEGEND AND AASHTO CLASSIFICATION | ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED. | WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED. | A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT |
| GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIALS | MINERALOGICAL COMPOSITION | CRYSTALLINE FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT | WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND |
| LLASS. (\$39% PASSING "200) (>30% PASSING "200) | MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE. | ROCK (CR) WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC. | SURFACE. |
| CROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 CLASS. A-1-6 A-1-6 A-1-6 A-2-4 A-2-5 A-2-6 A-2-7 A-3 A-4 A-5 A-6 A-7 | COMPRESSIBILITY | NON-CRYSTALLINE FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN | CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM |
| SYMBOL 000000000000000000000000000000000000 | SLIGHTLY COMPRESSIBLE LL < 31 | ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC. | OF SLOPE. |
| 7 PASCING. | MODERATELY COMPRESSIBLE LL = 31 - 50 HIGHLY COMPRESSIBLE LL > 50 | COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED | CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. |
| *10 50 MX GRANULAR SIL1- MUCK, | PERCENTAGE OF MATERIAL | CP) SHELL BEDS, ETC. WEATHERING | DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT |
| *40 30 MX 50 MX 51 MN 51 MN 51 MX 51 MN 35 MX 35 MX 35 MX 35 MX 36 MN 36 | GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL | FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER | ROCKS OR CUTS MASSIVE ROCK. |
| MATERIAL | TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% | HAMMER IF CRYSTALLINE. | <u>DIP</u> - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. |
| PASSING *40 40 MX 41 MN | MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% | VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, (V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF | DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE |
| PI 6 MX NP 18 MX 18 MX 11 MN 11 MN 18 MX 18 MX 11 MN 11 MN MODERATE ORGANIC | HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE | OF A CRYSTALLINE NATURE. | LINE OF DIP, MEASURED CLOCKWISE FROM NORTH, |
| GROUP INDEX W W 4 MX 8 MX 12 MX 16 MX NU MX AMUUNIS UF SOILS | GROUND WATER | SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR | FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. |
| OF MA IOR GRAVEL AND FINE SILTY OR CLAYEY SILTY CLAYEY MATTER | ▼ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING | CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. | FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. |
| MATERIALS SAND SAND GRAVEL AND SAND SOILS SOILS | STATIC WATER LEVEL AFTER 24 HOURS | MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS | FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. |
| GEN. RATING EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE | | DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED | FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. |
| PI OF A-7-5 SUBGROUP IS ≤ LL - 30; PI OF A-7-6 SUBGROUP IS > LL - 30 | SPRING OR SEEP | WITH FRESH ROCK. MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, IN GRANITOID ROCKS, ALL FELDSPARS DULL | FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE |
| CONSISTENCY OR DENSENESS | MISCELLANEOUS SYMBOLS | SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH | FIELD. |
| PRIMARY SOIL TYPE COMPACTNESS OR RANGE OF STANDARD RANGE OF UNCONFINED PENETRATION RESISTENCE COMPRESSIVE STRENGTH | ROADWAY EMBANKMENT (RE) 25/025 DIP & DIP DIRECTION | (MOD. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK, ROCK GIVES 'CLUNK' SOUND WHEN STRUCK, IF TESTED, WOULD YIELD SPT REFUSAL | JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO |
| CONSISTENCY (N-VALUE) (TONS/FT ²) | WITH SOIL DESCRIPTION → OF ROCK STRUCTURES | SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT | ITS LATERAL EXTENT. |
| GENERALLY VERY LOOSE | SOIL SYMBOL SOIL SYMBOL SPET DOPT DATE TEST BORING SLOPE INDICATOR INSTALLATION | (SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. | LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. |
| MATERIAL MEDIUM DENSE 10 TO 30 N/A | M | IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF | MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. |
| DENSE 30 TO 50 | THAN ROADWAY EMBANKMENT AUGER BORING CONE PENETROMETER | VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK | PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE |
| VERY SOFT < 2 < 0.25 | — INFERRED SOIL BOUNDARY — CORE BORING SOUNDING ROD | (V SEV.) REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR | OF AN INTERVENING IMPERVIOUS STRATUM. |
| GENERALLY SOFT 2 TO 4 0.25 TO 0.5 SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0 | INFERRED ROCK LINE MN MONITORING WELL TEST BORING | VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES < 100 BFF</u> COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND | RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. |
| MATERIAL STIFF 8 TO 15 1 TO 2 (COHESIVE) VERY STIFF 15 TO 30 2 TO 4 | A DIEZOMETED | SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE. | ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE |
| HARD > 30 > 4 | ***** ALLUVIAL SOIL BOUNDARY \(\triangle \) INSTALLATION \(\triangle \) SPT N-VALUE | ROCK HARDNESS | RUN AND EXPRESSED AS A PERCENTAGE. |
| TEXTURE OR GRAIN SIZE | RECOMMENDATION SYMBOLS | VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES | SAPPOLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. |
| U.S. STD. SIEVE SIZE 4 10 40 60 200 270 OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053 | UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE | SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. | SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO |
| COADCE | SHALLOW UNDERCUT UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL | HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN. | THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. |
| BOULDER COBBLE GRAVEL SAND SAND SILT CLAY | | MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE | SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT |
| GRAIN MM 305 75 2.0 0.25 0.05 0.005 | ABBRE VIATIONS AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST | HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. | OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF |
| SIZE IN. 12 3 | BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED | MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. | A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL |
| SOIL MOISTURE - CORRELATION OF TERMS | CL CLAY MOD MODERATELY 7 - UNIT WEIGHT CPT - CONE PENETRATION TEST NP - NON PLASTIC 7 - DRY UNIT WEIGHT | HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK. | WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. |
| SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION | CSE COARSE ORG ORGANIC | SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS | STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY |
| (HITERDERU LIMITS) DESCRIPTION | DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK | FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE. | TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL |
| - SATURATED - USUALLY LIQUID; VERY WET, USUALLY (SAT.) FROM BELOW THE GROUND WATER TABLE | e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON F - FINE SL SILT, SILTY ST - SHELBY TUBE | VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH | LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. |
| PLASTIC LIQUID LIMIT | FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK | SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE, CAN BE SCRATCHED READILY BY FINGERNAIL. | TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER. |
| RANGE - WET - (W) SEMISOLID; REGUIRES DRIVING TO | FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL FRAGS FRAGMENTS w - MOISTURE CONTENT CBR - CALIFORNIA BEARING | FRACTURE SPACING BEDDING | BENCH MARK: |
| (PI) PL PLASTIC LIMIT ATTAIN OPTIMUM MOISTURE | HI HIGHLY V - VERY RATIO | TERM SPACING TERM THICKNESS | DENGTI FIRM |
| OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE | EQUIPMENT USED ON SUBJECT PROJECT | VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET | ELEVATION: FEET |
| SL _ SHRINKAGE LIMIT | DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE: X CME-45B CLAY BITS AUTOMATIC MANUAL | MODERATELY CLOSE | NOTES: |
| - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE | 6' CONTINUOUS ELIGHT AUGER | VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET | U.C.P. = UNDIFFERENTIATED COASTAL PLAIN |
| | X DIEDRICH D-50 X 8*HOLLOW AUGERS CORE SIZE: -BH | THINLY LAMINATED < 0.008 FEET INDURATION | W/ = WITH |
| PLASTICITY PLASTICITY INDEX (PL) PDV STDENICTU | | FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. | - = APPROXIMATE HORIZONTAL LIMIT OF ORGANIC SOIL |
| PLASTICITY INDEX (PI) DRY STRENGTH NON PLASTIC 0-5 VERY LOW | TUNGCARBIDE INSERTS | RUBBING WITH FINGER FREES NUMEROUS GRAINS; | |
| SLIGHTLY PLASTIC 6-15 SLIGHT MODERATELY PLASTIC 16-25 MEDIUM | VANE SHEAR TEST CASING W/ ADVANCER HAND TOOLS: | GENILE BLUW BY HAMMER DISINTEGRATES SAMPLE. | |
| HIGHLY PLASTIC 26 OR MORE HIGH | PORTABLE HOIST TRICONE STEEL TEETH X HAND AUGER | MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER. | |
| COLOR | TRICONE TUNGCARB. SOUNDING ROD | INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; | |
| DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). | CORE BIT SOSSIENT TEST | DIFFICULT TO BREAK WITH HAMMER. | |
| MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE. | | EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS. | DATE: 8-15-1 |
| | | I | |

STATE OF NORTH CAROLINA N.C. 3 18 U-4902D DIVISION OF HIGHWAYS STATE PROJ. NO. 40238.1.1 4902D NEW HANOVER COUNTY LOCATION: MARKET STREET FROM JUST WEST OF US 74 (MLK PARKWAY) TO WEST OF STATION ROAD AND WEST OF WENDOVER LN TO MARSH OAKS DR TYPE OF WORK: GRADING, PAVING, AND DRAINAGE PROJEC -L- STA. 288 + 00.00 END TIP PROJECT U-4902D END CONSTRUCTION -L- STA. 203 + 50.00 BEGIN TIP PROJECT U-4902D **BEGIN CONSTRUCTION VICINITY MAP** -L- STA. 202 + 50.00-L- STA. 135 + 00.00 END TIP PROJECT U-4902C **END CONSTRUCTION** WILMINGTON CITY LIMITS **L**- STA. 136 + 85.00 TO US 117 INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION L- STA. 24 + 75.00 BEGIN TIP PROJECT U-4902C 1. CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II . PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION **BEGIN CONSTRUCTION** 2. A PORTION OF THIS PROJECT IS WITHIN THE MUNICIPAL BOUNDARIES OF THE CITY OF WILMINGTON. HYDRAULICS ENGINEER DESIGN DATA PROIECT LENGTH Prepared in the Office of: **GRAPHIC SCALES** HNTB NORTH CAROLINA, P.C. ADT 2008 = 52,900343 E. Six Forks Road, Raleigh NC, 27609 LENGTH OF ROADWAY T.I.P. PROJECT U-4902C/D = 3.688 MILES ADT 2035 = 66,0002012 STANDARD SPECIFICATIONS DAVID BASS, PE PROJECT ENGINEER RIGHT OF WAY DATE: TOTAL LENGTH OF T.I.P. PROJECT U-4902C/D = 3.688 MILES ROADWAY DESIGN V = 50 MPH**SEPTEMBER 10, 2015 ENGINEER** PROFILE (HORIZONTAL) TRACE HOWELL, EI * TTST = 2% DUAL 4% FUNC CLASS = PRINCIPAL ARTERIAL LETTING DATE: AUGUST 16, 2016 ALL LENGTH BASED ON -L- CENTERLINE DAVID LEONARD, PE **REGIONAL TIER**

STATE PROJECT: 40238.1.4 (U-4902D)

F.A. PROJECT: NHS-0017(76) COUNTY: NEW HANOVER

DESCRIPTION: US 17 Business (Market Street) from Lendire Road to Marsh Oaks Drive

SUBJECT: Geotechnical Inventory Report

PROJECT DESCRIPTION

The proposed project is located in New Hanover County just north of the city of Wilmington. The proposed roadway will consist of widening existing US 17 Business (Market Street).

A geotechnical field investigation was conducted by CATLIN Engineers and Scientists (CATLIN) personnel in March through May 2016. Standard penetration test borings were advanced using hollow-stem augers under the direct supervision of a North Carolina Licensed Well Contractor with a Central Mine Equipment (CME) CME 45B drilling rig and a Diedrich D-50 drilling rig equipped with an automatic hammer. Standard penetration testing was conducted in general accordance with American Society for Testing and Materials (ASTM) D-1586-84, "Penetration Test and Split Barrel Sampling of Soils" or American Association of State Highway and Transportation Officials (AASHTO) Standard Method T206-81. Hand auger borings were advanced across the entire project site to augment the SPT data. A total of 50 borings totaling roughly 355 vertical feet were advanced during this investigation.

Representative soil samples were collected for visual classification in the field and selected samples were submitted for laboratory analysis by CATLIN Geotechnical Laboratory in Wilmington, North Carolina. Samples were prepared and analyzed in accordance with one or more of the following AASHTO Standards as modified by NCDOT:

| T 87-86 | (Dry Preparation of Disturbed Soil) |
|----------|-------------------------------------|
| T 88-93 | (Particle Size Analysis) |
| T 89-94 | (Liquid Limit) |
| T 90-94 | (Plastic Limit) |
| T 265-93 | (Soil Moisture Content) |
| T 267 | (Organic Content) |

CATLIN personnel used Global Positioning System (GPS) technology to locate all borings. All horizontal locations were recorded to the nearest foot and are presented in the North Carolina State Plane (NCSP), North American Datum 1983 (NAD 83). Vertical control was measured to the nearest 0.1 foot and referenced to the National Geodetic Vertical Datum

1988 (NGVD88). All measurements were recorded and reported in United States Survey Feet (US Ft).

The following alignments were investigated. Plan sheets, subsurface profiles, and selected cross sections of the alignment are included in this report.

| <u>Line</u> | Station (±) |
|-------------|------------------|
| -L- | 202+50 to 288+00 |
| -Y28A- | 10+00 to 11+25 |
| -Y29- | 13+00 to 17+10 |
| -Y30- | 10+00 to 11+75 |
| -Y31- | 11+25 to 12+50 |
| -Y32- | 11+50 to 13+02 |
| -Y33- | 11+65 to 12+50 |

The project reportedly will consist of 1.734 miles of roadway widening.

AREAS OF SPECIAL GEOTECHNICAL INTEREST

1) <u>GROUNDWATER</u>: Groundwater within six (6) feet of natural ground was noted at the following sections:

| <u>Line</u> | Station (±) |
|-------------|------------------|
| -L- | 202+50 to 224+00 |
| -L- | 226+00 to 254+00 |
| -L- | 272+00 to 288+00 |
| -Y28A- | 10+00 to 11+25 |
| -Y29- | 13+00 to 17+10 |
| -Y30- | 10+00 to 11+75 |
| -Y31- | 11+25 to 12+50 |
| -Y32- | 11+50 to 13+02 |

2) <u>COHESIVE SOILS</u>: Clay or cohesive soils which may have the potential to cause embankment/subgrade and or slope stability problems during construction was encountered at the following sections of the project:

| <u>Line</u> | Station ± | |
|-------------|------------------|--|
| -l - | 220+14 to 221+40 | |

3) ORGANIC SOILS: Organic material that may cause construction related issues was identified at the following sections on the project:

| <u>Line</u> | Station ± |
|-------------|------------------|
| -L- | 202+50 to 204+03 |
| -L- | 208+00 to 209+50 |
| -L- | 221+29 to 222+40 |
| -L- | 242+00 to 244+60 |
| -L- | 271+00 to 275+00 |
| -L- | 280+40 to 281+70 |
| -Y28A- | 10+00 to 11+25 |

4) <u>WATER WELLS</u>: No water wells were identified within the proposed construction limits. Water supply wells may be present along the project corridors that were not detected.

PHYSIOGRAPHY AND GEOLOGY

The project is located within the eastern most portion of the North Carolina Coastal Plain physiographic province. Geology in the vicinity of Wilmington is dominated by Undifferentiated Coastal Plain (U.C.P.) materials which are noted as Quaternary Surficial Deposits on the Geologic Map of North Carolina. Coastal Plain materials are described as sand, clay, gravel, and peat deposits which were deposited in marine, fluvial, eolian, and lacustrine environments. Sediments of the Castle Hayne and Peedee Formations are reported to underlay the U.C.P. deposits in the vicinity of Wilmington.

Land use in the area is primarily commercial and residential. The land surface in the project vicinity is dominated by flat terrain typical of coastal environments with land surface elevations ranging from approximately 25 to 47 feet. Numerous underground and overhead utilities exist in the vicinity of the proposed project. The project is primarily drained by surficial runoff to stormwater systems and drainage ditches.

GROUNDWATER

Groundwater data was collected from open boreholes, where possible, during the field investigation conducted between March and May 2016

Measured groundwater elevations (24 hour measurements) ranged from elevation 22.5 feet to 42.5 feet with an average elevation of 34.8 feet. Twenty-four hour depth to groundwater measurements ranged from 1.5 feet to 6.4 feet below existing land surface (BLS), with an average depth to water of 3.7 feet. Formational material in which groundwater was typically observed was found to be predominantly silty sand material with an assumed moderate permeability.

SOIL PROPERTIES

Soils encountered at the project site include roadway embankment and undifferentiated coastal plain sediments.

Roadway embankment soils were identified beneath and adjacent to existing roadways and consist of loose to medium dense, sand and silty sand (A-3 and A-2-4).

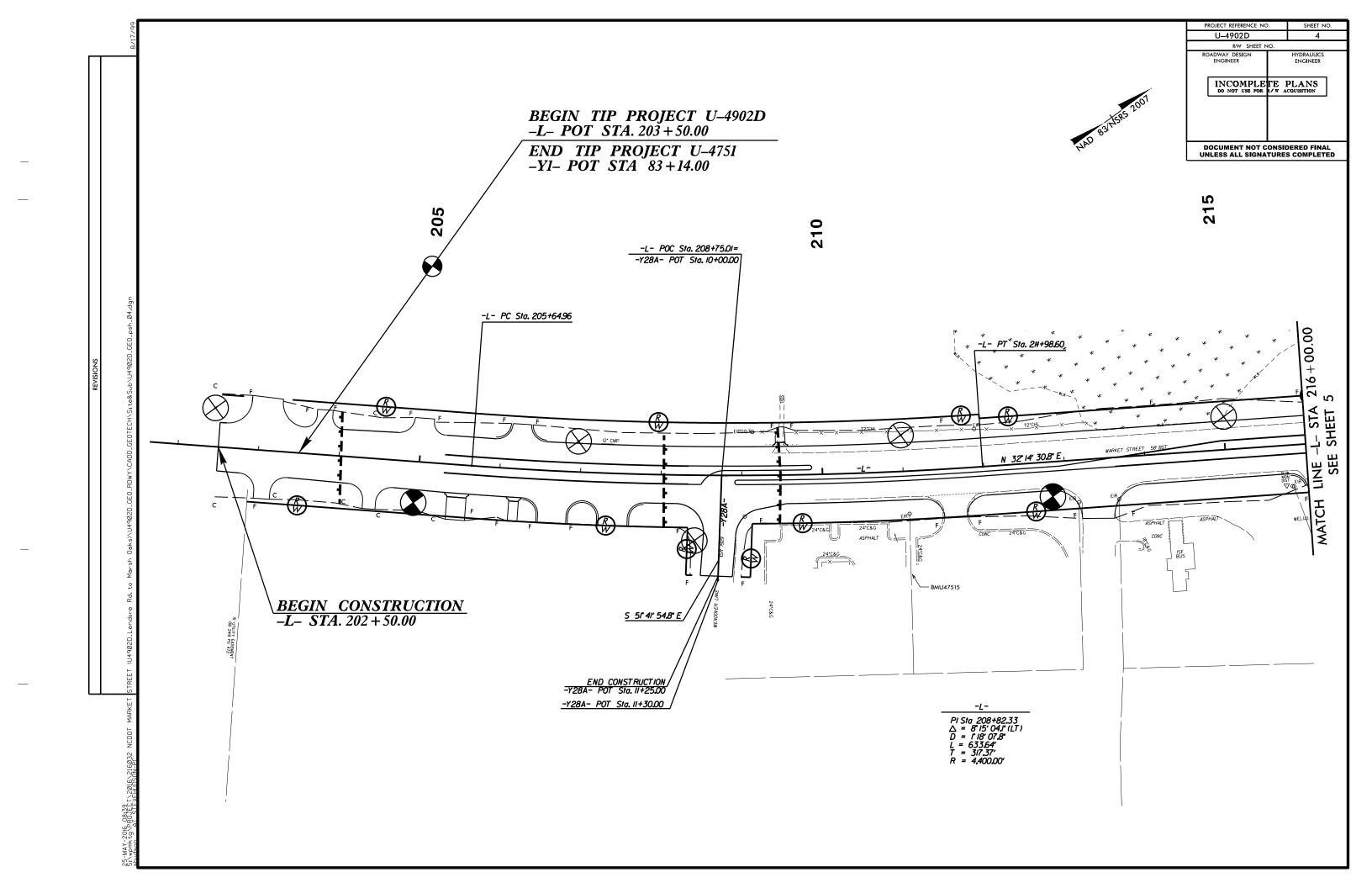
The dominant U.C.P. material encountered consisted of very loose to medium dense sand to silty sand (A-3 and A-2-4). The sand was encountered from land surface, or below the roadway embankment to a depth of at least 12 feet BLS (depth of deepest borings). A small amount of highly plastic clay, silt, and organic sands were encountered at the previously described locations.

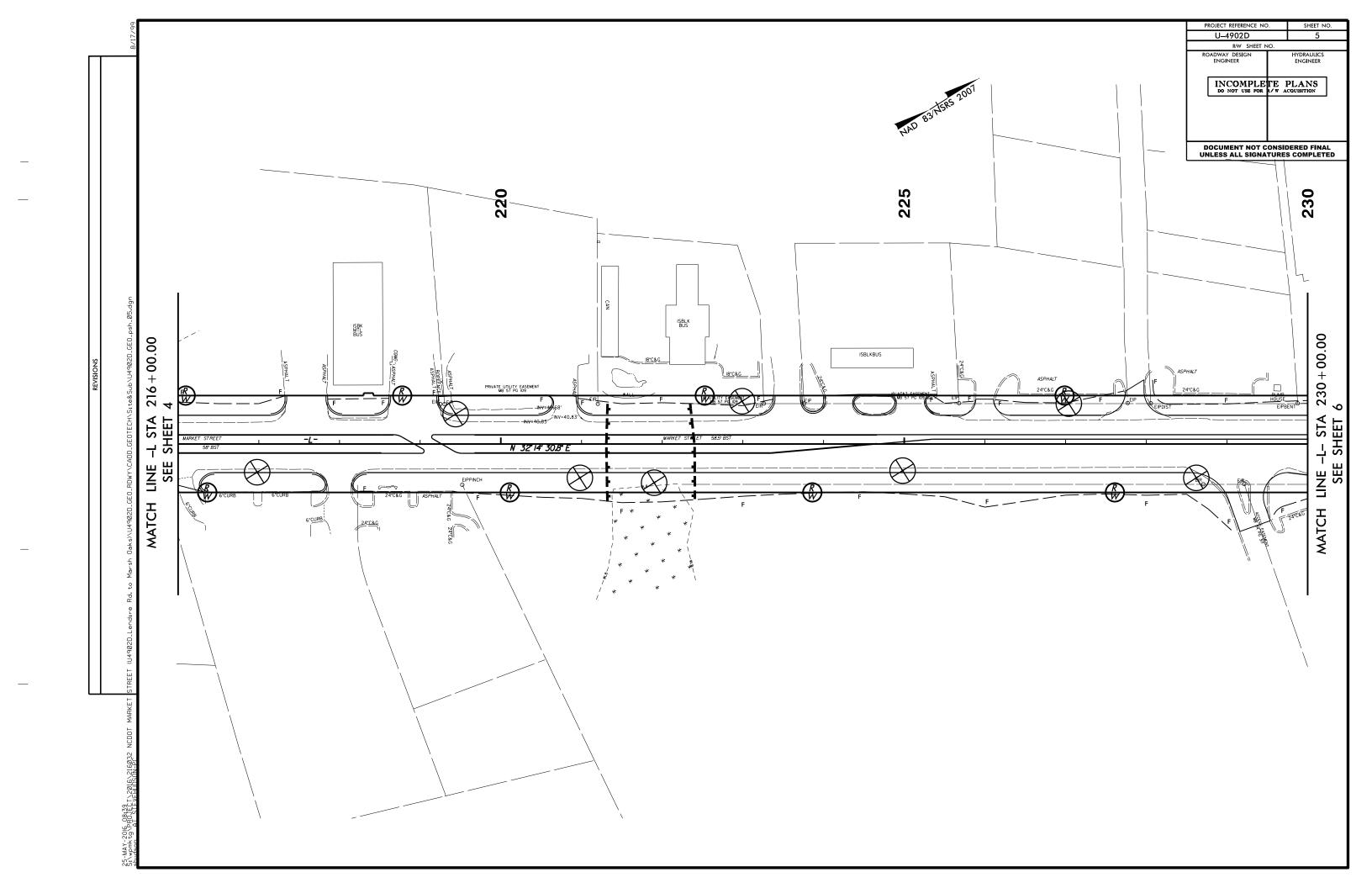
Material within the identified organic soil area was described as very loose, dark brown to black, highly organic (15.2% to 18.9%) sand to silty fine sand with little (3.1% to 6.8%) organics.

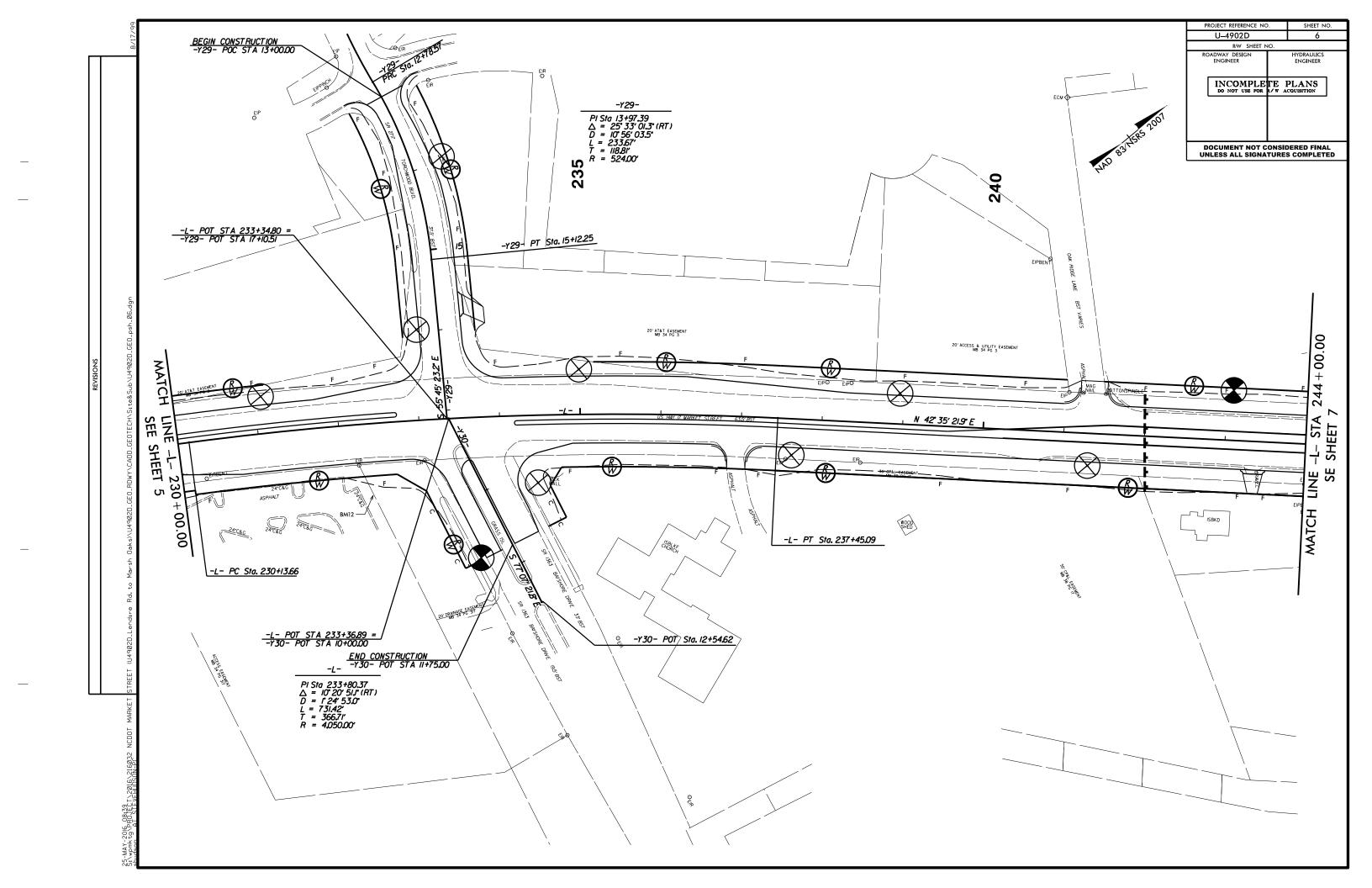
Prepared by,
Steven V. Hudson, L.G.
Project Geologist

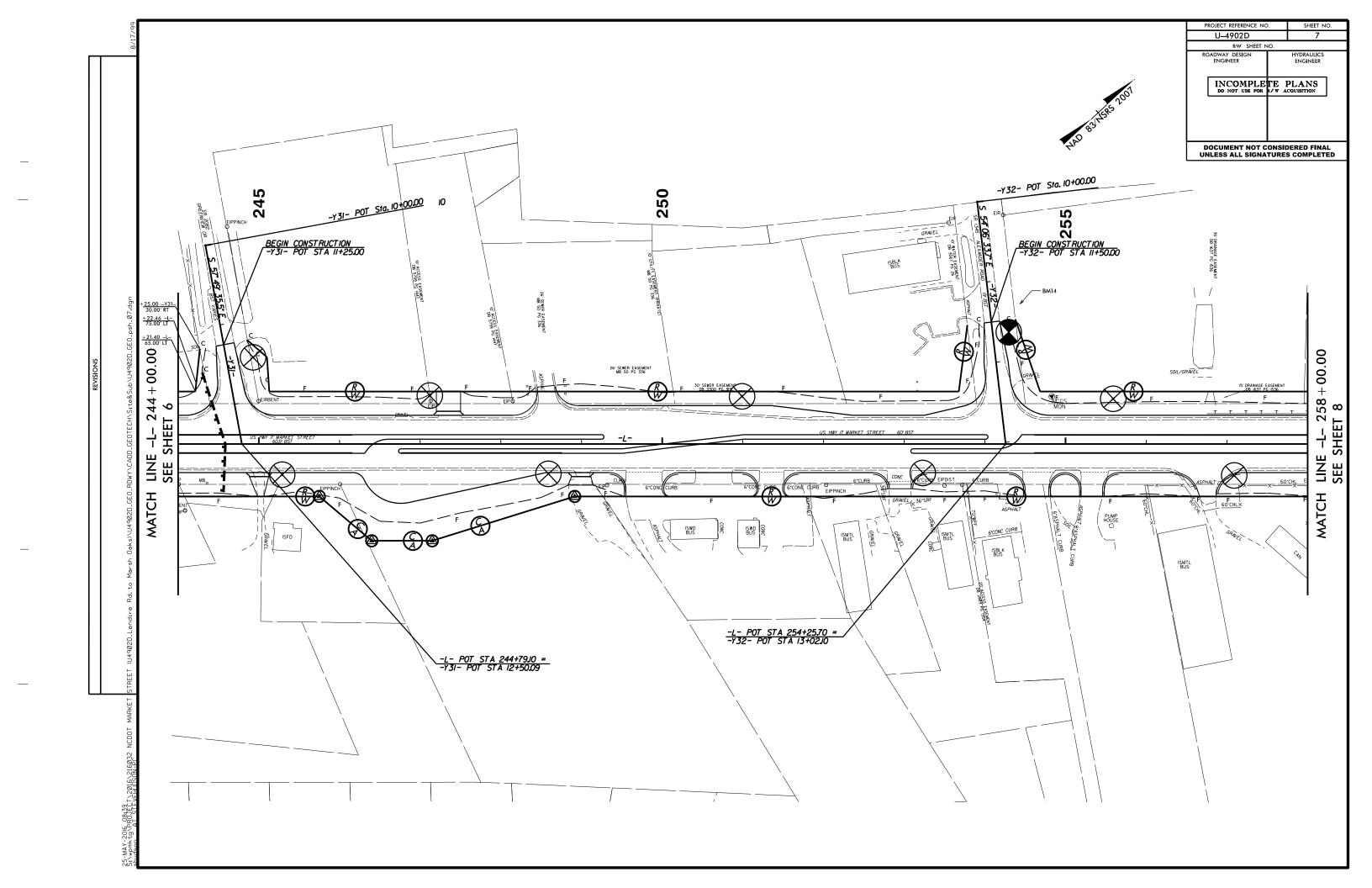
State

5/31/2016

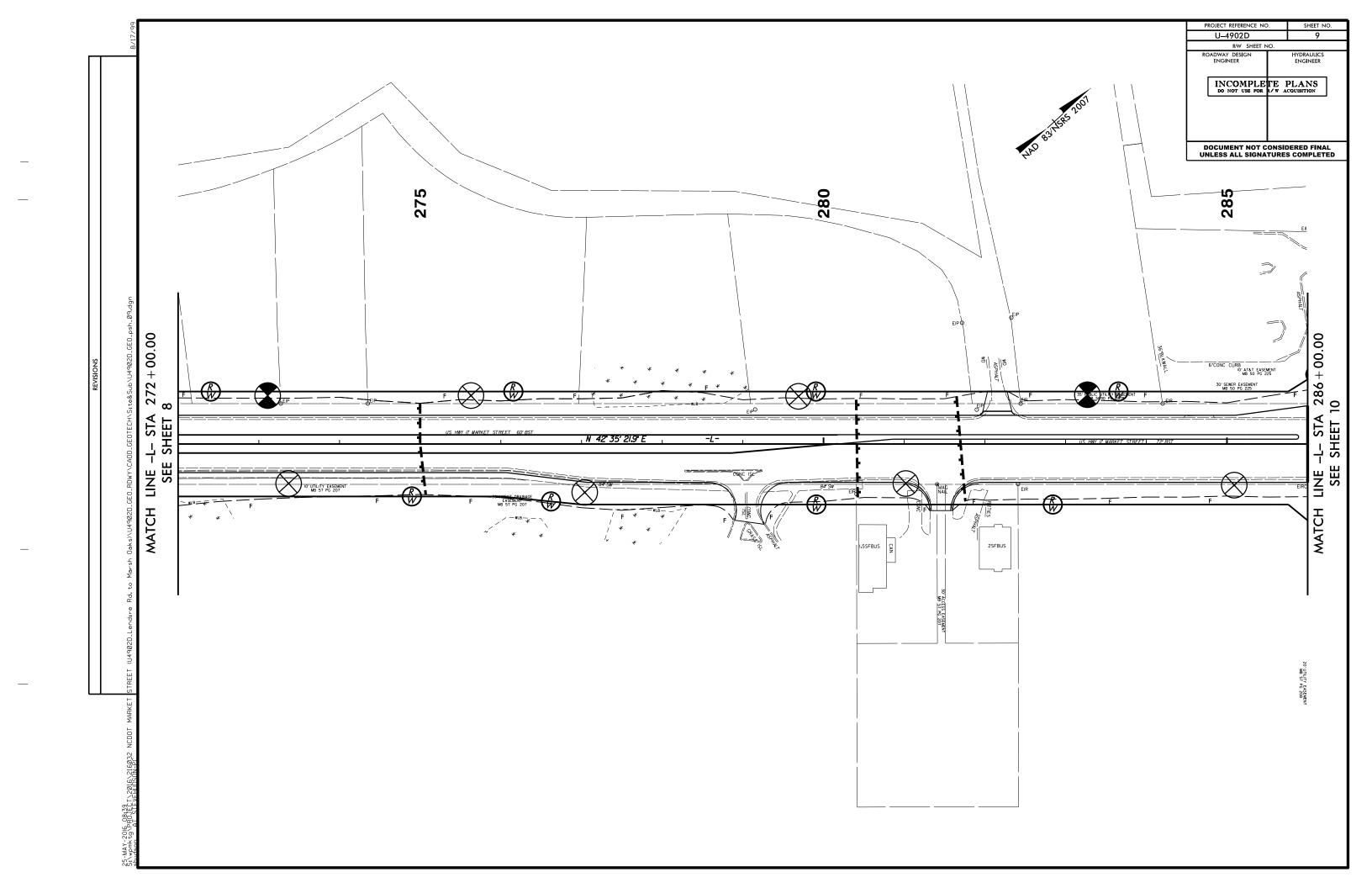


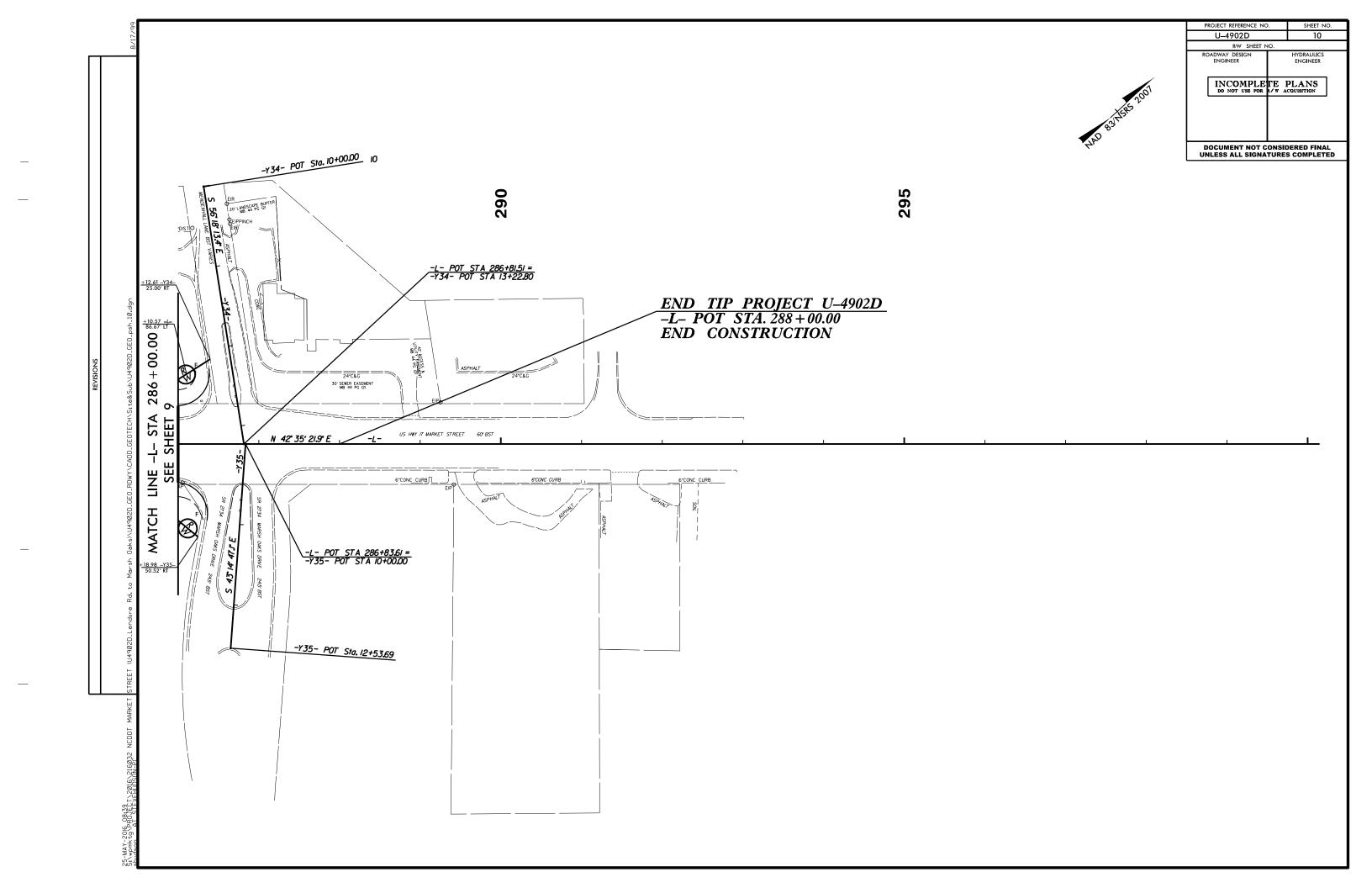


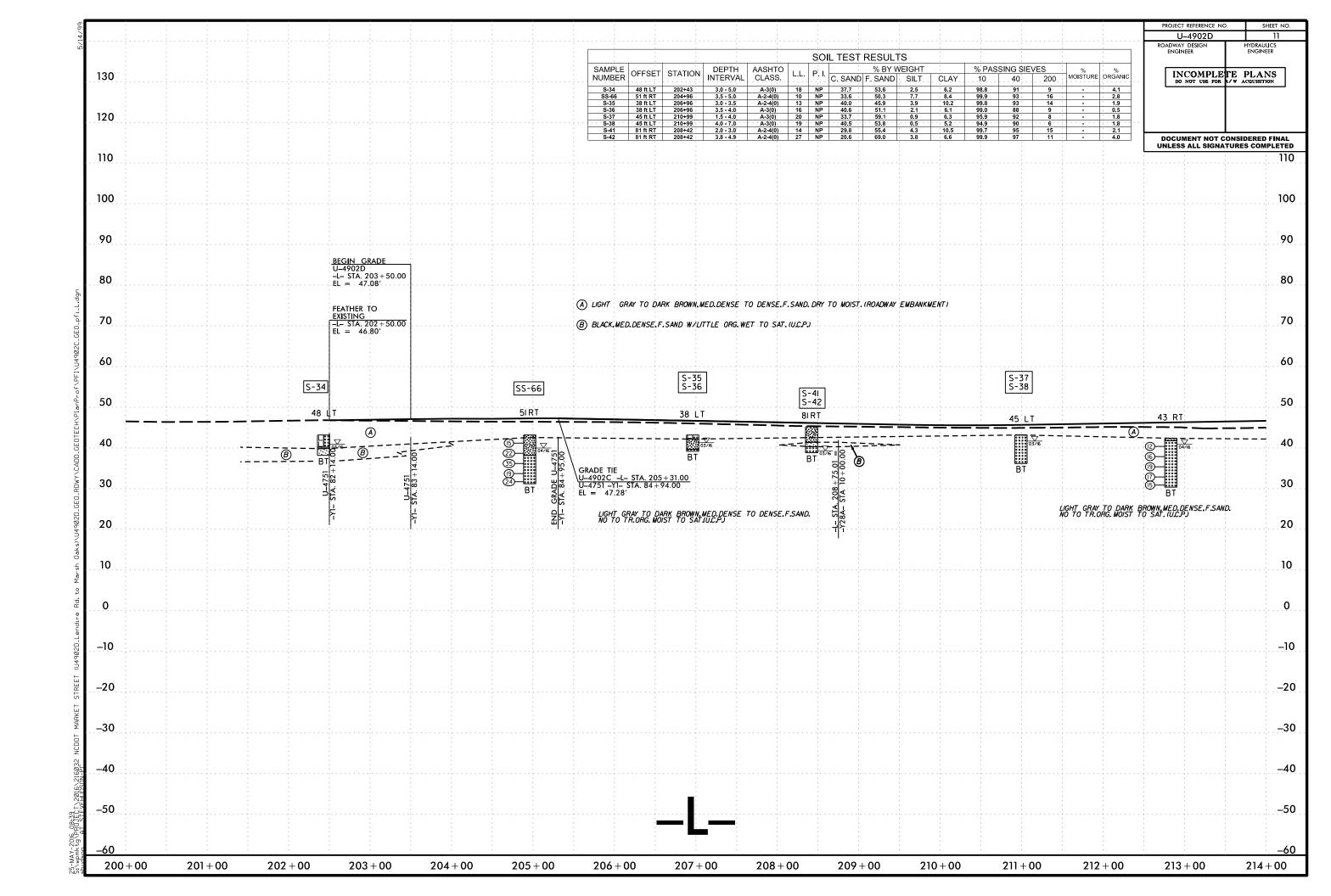


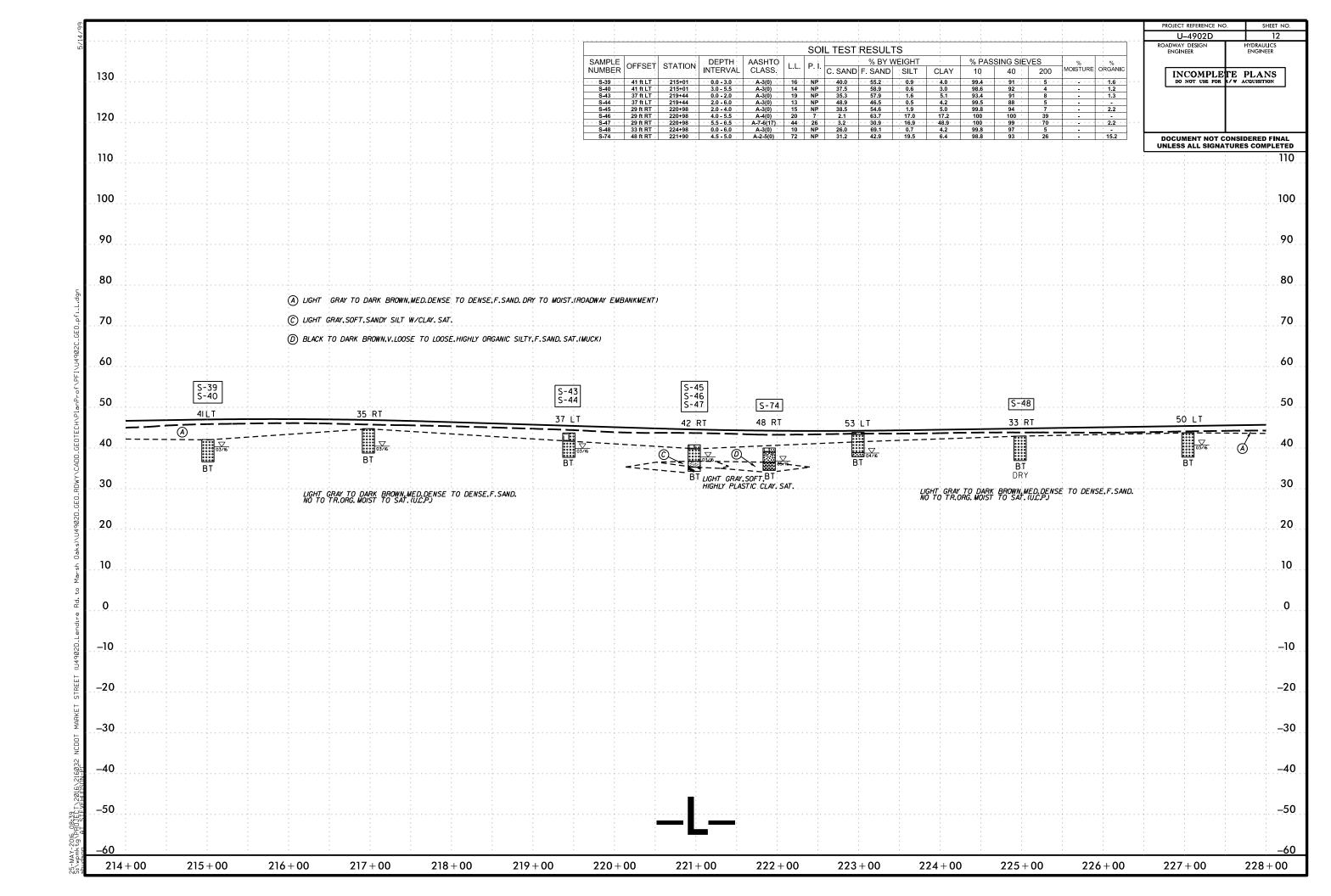


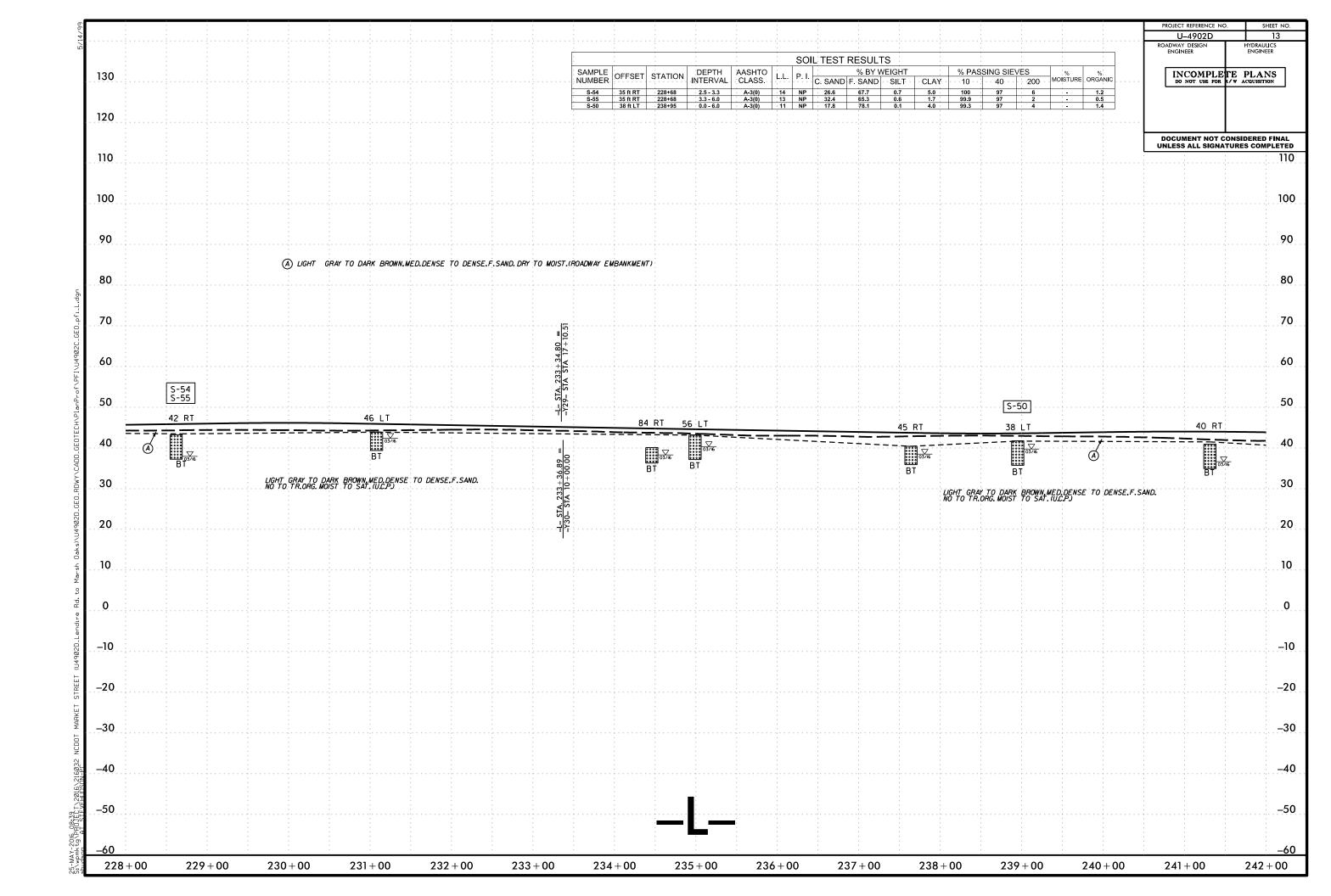
PROJECT REFERENCE NO. SHEET NO. U-4902D R/W SHEET NO. ROADWAY DESIGN ENGINEER HYDRAULICS ENGINEER INCOMPLETE PLANS
DO NOT USE FOR TW ACQUISITION DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED 260 270 265 -Y33- POT Sta. 10+00.00 10 MATCH LINE -L- STA 258+00.00 SEE SHEET 7 MATCH LINE -L- STA 272+00.00 SEE SHEET 9 -L- POT STA 269+08.94 = -Y33- POT STA 12+50.70

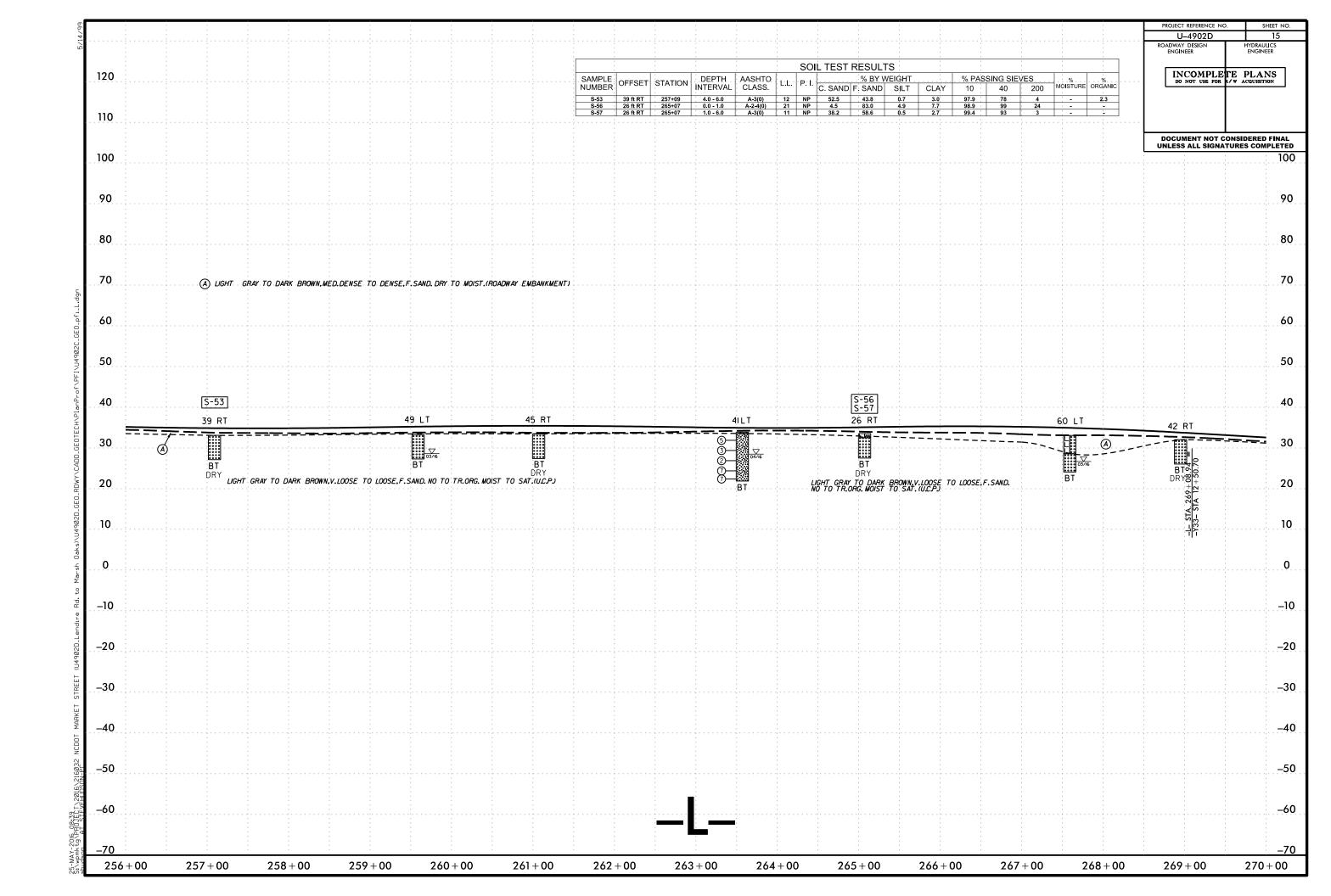


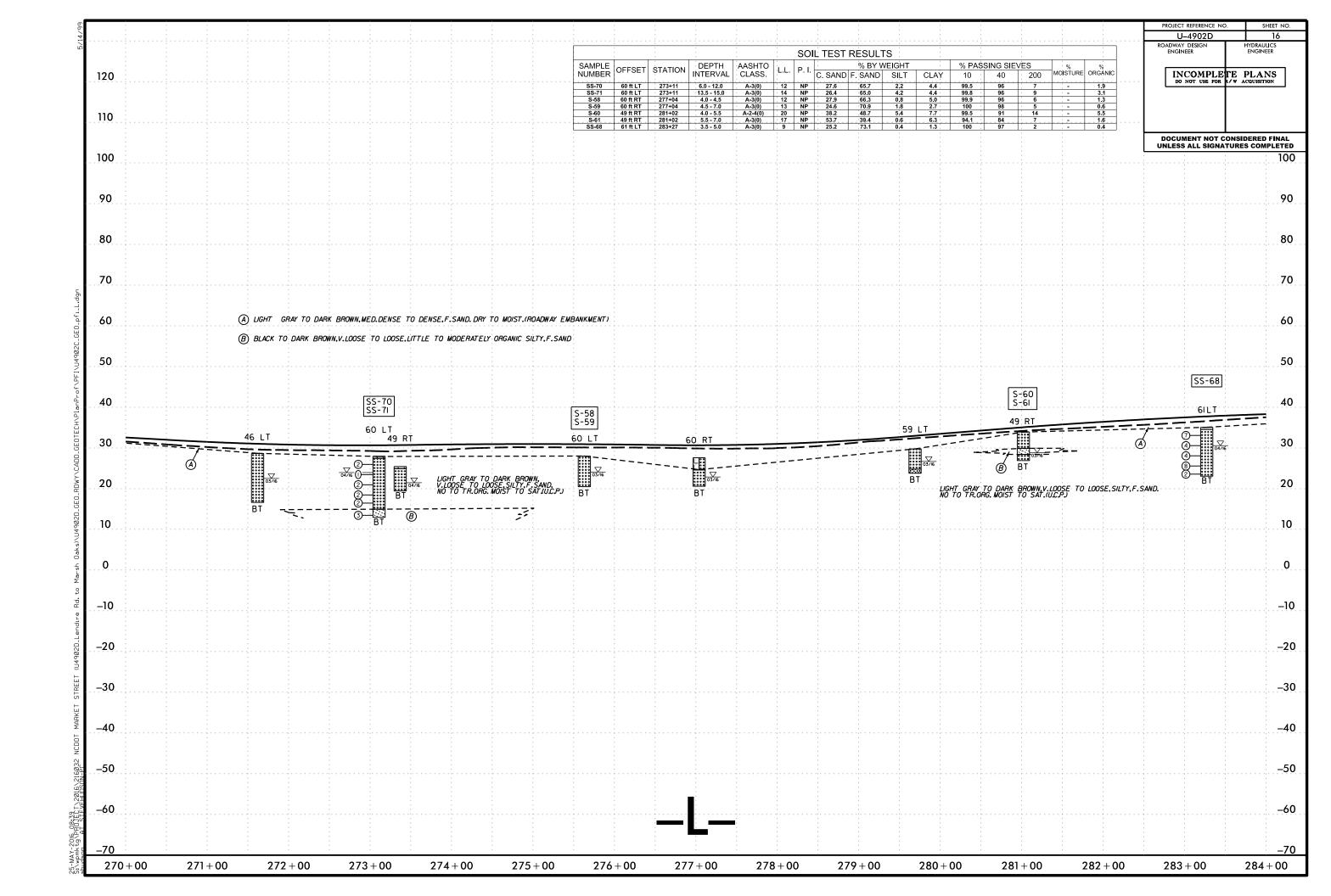


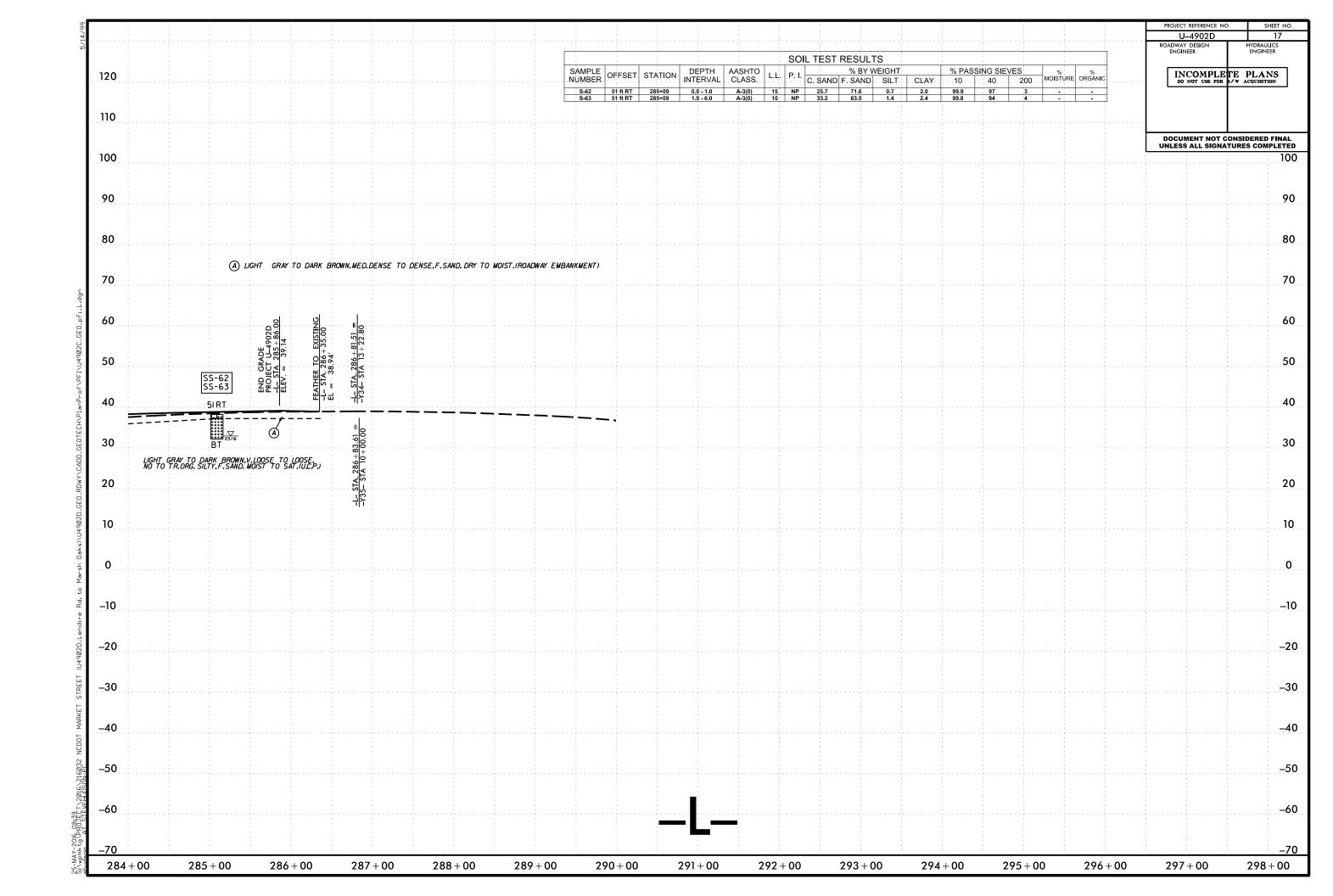


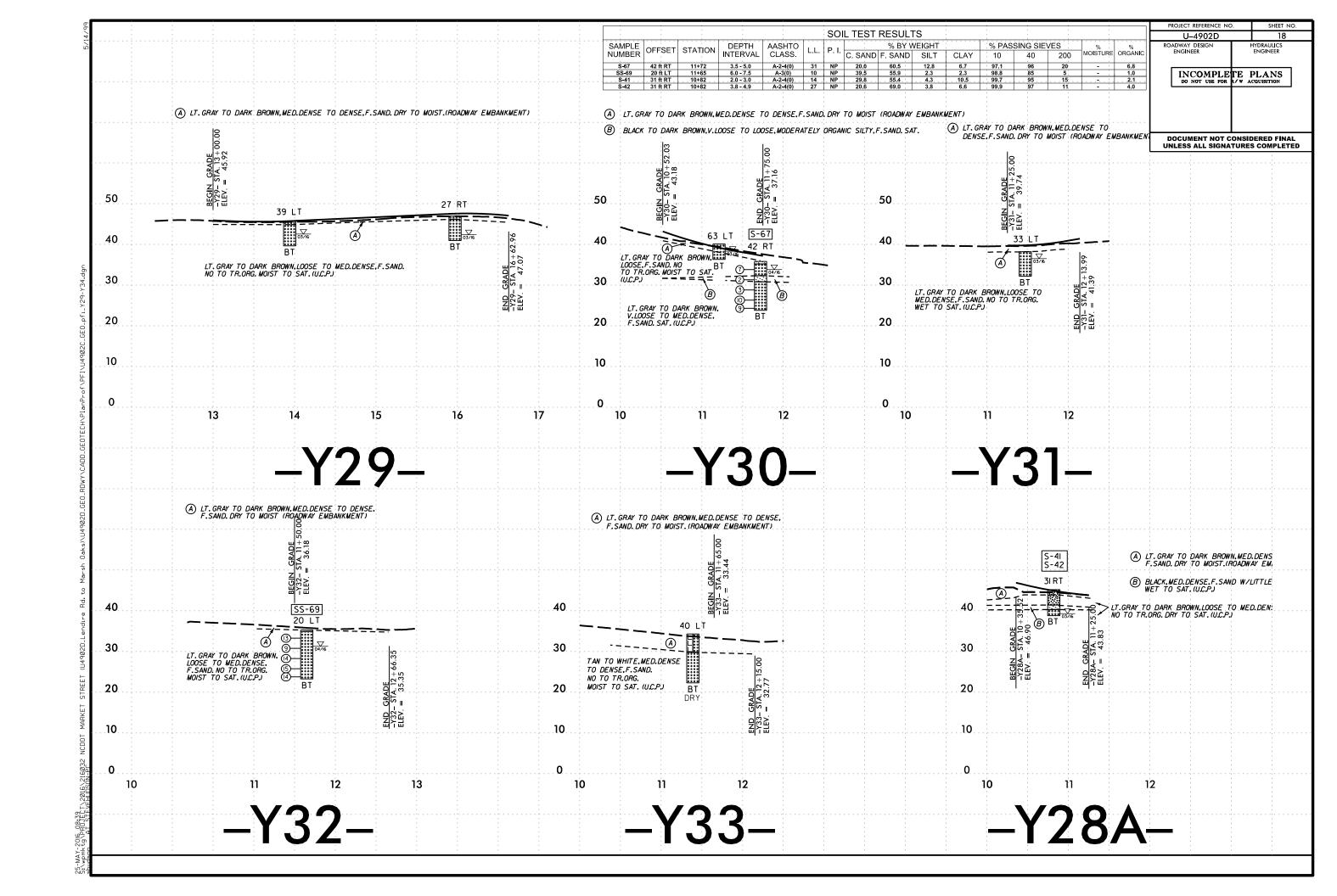














Telephone: (910) 452-5861 Fax: (910) 452-7563

www.catlinusa.com

May 15, 2017

MEMORANDUM TO: John L. Pilipchuk, LG, PE

State Geotechnical Engineer

FROM: Ben D. Lackey Jr, PE

CATLIN Senior Geotechnical Engineer

STATE PROJECT: 40238.1.4 (U-4902D)

F.A. NUMBER: NHS-0017(76) COUNTY: NEW HANOVER

DESCRIPTION: US 17 Business (Market Street) from Lendire Road to Marsh Oaks

Drive

SUBJECT: Geotechnical Report Addendum – Ground Improvement

CATLIN Engineers and Scientists (CATLIN) have completed a subsurface investigation and recommendations addendum to address the long term settlement and slope stability in the area listed below.

| <u>Line</u> | Station +/- |
|-------------|------------------|
| | |
| -L- | 277+47 to 279+77 |

CATLIN recommends the use of lightweight aggregate embankment with high strength fabric for embankment stabilization for the proposed roadway embankment and as a replacement for the existing roadway embankment in this area.

1. Fabric for Embankment Stabilization

The fabric for the embankment stabilization shall be installed in the following area, as shown on the detail plan, prior to placement of the lightweight aggregate embankment fill. Fabric for embankment construction shall meet the requirements of the attached "Fabric for Embankment Stabilization" special provision. Recommend a quantity of 3,650 square yards of Fabric for Embankment Stabilization in the contract.

| <u>Line</u> | Station +/- |
|-------------|------------------|
| | |
| -L- | 277+47 to 279+77 |

2. Removal of the existing Roadway Embankment

The existing roadway embankment shall be removed from the following area.

| <u>Line</u> | Station +/- |
|-------------|------------------|
| | |
| -L- | 277+47 to 279+77 |

Recommend 3,100 cubic yards of suitable unclassified excavation be included in the contract. Use of suitable unclassified excavation material shall meet the requirements of the attached "Special Handling of Unclassified Excavation" special provision.

3. Lightweight Aggregate

Lightweight aggregate shall be used to construct the proposed embankment to subgrade from -L-Station $277+47 \pm$ to $279+77 \pm$. Lightweight aggregate for construction shall meet the requirements of the attached "Lightweight Aggregate" special provision. Include a quantity of 3,000 tons of Lightweight Aggregate in the contract.

4. Shoulder and Slope Material

The fill slope material should be placed to a height of 1 foot on the roadway side slopes which are formed with lightweight aggregate. Material for fill slope shall meet the requirements of the attached "Shoulder and Fill Slope Material" project special provision. Recommend 275 cubic yards of borrow excavation should be included in the contract.

5. Fabric for Soil Stabilization

The fabric for soil stabilization shall be installed between the lightweight aggregate embankment. Include 7,300 square yards of Fabric for Soil Stabilization in the contract to be used in the following sections, as shown on the detail plan.

| <u>Line</u> | Station +/- |
|-------------|------------------|
| | |
| -L- | 277+47 to 279+77 |

5. Attachments

- Quantity Summary
- Special Provisions
- 1. Fabric for Embankment Stabilization
- 2. Lightweight Aggregate
- 3. Shoulder and Fill Slope Material
- 4. Special Handling of Unclassified Excavation
 - Plan Sheet
- 1. Lightweight Aggregate Embankment Detail Plan (1 sheet)

Prepared By:



Lee Stone, P.G. CATLIN Senior Geologist



Ben D. Lackey Jr. P.E. CATLIN Geotechnical Engineer

40238.1.4 (U-4902D) Page 4 of 13

Quantity Summary

| <u>Item Number</u> <u>Description</u> | | <u>Quantity</u> | Comment | | |
|---------------------------------------|--|-----------------|---------------|--|--|
| | Fabric for Embankment Stabilization (SP) | 3,650 SY | See Section 1 | | |
| | Special Handling of Unclassified Excavation (SP) | 3,100 CY | See Section 2 | | |
| | Lightweight Aggregate (SP) | 3,000 TON | See Section 3 | | |
| 0106000000-E | Borrow Excavation for Shoulder and Fill Slope Material | 275 CY | See Section 4 | | |
| 0196000000-E | Fabric for Soil Stabilization | 7,300 SY | See Section 5 | | |

FABRIC FOR EMBANKMENT STABALIZATION

DESCRIPTION:

This work consists of furnishing and installing synthetic fabric for stabilizing embankment in accordance with this provision or as directed by the Engineer. The work shall include maintaining the fabric in the required configuration until completion and acceptance of overlying work items. The fabric shall be placed at the locations shown in the plans or as directed by the Engineer.

MATERIAL:

The fabric material for embankment stabilization shall be made of high-tenacity polyester in the machine direction with a plain straight weave pattern and polyester or polypropylene in the cross direction or approved equal. The fabric shall be composed of strong rot-proof synthetic fibers formed into a fabric of the woven type. The fabric shall be free of any treatment or coating which might significantly alter its physical properties after insulation.

The fabric shall contain stabilizers and/or inhibitors to make the filaments resistant to deterioration resulting from ultraviolet or heat exposure. The fabric shall be a pervious sheet of synthetic fibers oriented into a stable network so that the fibers retain their relative positions with respect to each other. The edges of the fabric shall be furnished to prevent the outer yarn from pulling away from the fabric. The fabric shall be free of defects or flaws which significantly affect its physical and/or filtering properties. Sheets of fabric shall be sewn together with a seam that furnishes the required minimum strengths. The seam thread shall be made of synthetic fibers which are resistant to deterioration, as are the fabric fibers. Lamination of fabric sheets to produce the physical requirements of a fabric layer will not be accepted.

During all periods of shipment and storage, the fabric shall be wrapped in a heavy duty protective covering to protect the fabric from direct sunlight ultraviolet rays, mud, dust, dirt, and debris. The fabric shall not be exposed to temperatures greater than 140° F. After the protective wrapping has been removed, the fabric shall not be left uncovered under any circumstances for longer than one (1) week.

The fabric shall meet the following requirements:

All values represent minimum average roll values (any roll in a lot should meet or exceed the minimum values in this table).

| Fabric Property | Test Method | Requirements |
|---|----------------------------------|--------------------------------|
| Min. Puncture Strength | ASTM D-4833 | 135 lbs |
| Min. Bursting Strength | ASTM D-3786 | 600 psi |
| Trapezoid Tear | ASTM D-4533 | 135 lbs |
| AOS, US STD Sieve | ASTM D-4751 | 20-70 |
| Seams, Strength Cross Machine Direction Only | ASTM D-4884 | 500 lbs/ft |
| Ultraviolet (UV) % Strength Retained | ASTM D-4355 | 30% |
| Permeability | ASTM D-4491 | .002 in/sec |
| Tensile Strength at 5% Strain | ASTM D-4595 (Wide Strip Test) | Machine Direction 8000 lbs/ft |
| Ultimate Tensile Strength | ASTM D-4595 (Wide Strip Test) | Machine Direction 20000 lbs/ft |

The Contractor shall furnish certified reports by an approved independent testing laboratory with each shipment of material attesting that the fabric meets the requirements of this provision; however, the material shall be subject to inspection, test, or rejection by the Engineer at any time. The Contractor shall furnish the Engineer certified test reports by an approved independent testing laboratory attesting that the sewn seam provides the required strength.

The Contractor shall furnish and place over the embankment stabilization fabric as shown in the plan or as directed by the Engineer.

CONSTRUCTION METHODS:

The fabric for embankment stabilization shall be placed at locations shown in the plans or as directed by the Engineer. The locations should be cleared and free of obstructions, debris, and pockets. Stumps shall be cut smooth at the ground elevation with the root system left intact. At the time of installation, the fabric shall be rejected if it had defects, rips, holes, flaws, deterioration, or damage incurred during manufacture, transportation, or storage.

The fabric for embankment stabilization shall be placed with the machine direction as shown on the plans or as directed by the Engineer. Fabric shall be laid smooth and free from tension, stress folds, wrinkles, or creases. All joints parallel to the machine direction shall be sewn by an approved method to develop the required seam strength. All sewn seams shall be placed facing upwards to allow for inspection. No seams will be allowed perpendicular to the machine direction. The lightweight aggregate should be placed and compacted using low ground pressure equipment as directed by the Engineer. All fabric which is damaged as a result of installation will be required to be replaced or repaired at the direction of the Engineer with no additional cost to the Department. Compaction equipment shall be such that it will not harm the fabric.

A lightweight aggregate layer at a depth shown in the plan or as directed by the Engineer shall be placed over the geosynthetic layer. End dumping fill directly on the fabric is not permitted. Spreading and compaction of lightweight aggregate with low ground pressure (<4 psi) equipment is required until 2 feet of lightweight fill has been placed.

Any fabric which is damaged as a result of installation or which is left uncovered for longer than one week after placement shall be replaced at no additional cost to the Department.

METHOD OF MEASUREMENT

The quantity of fabric to be paid for will be the number of square yards of "Fabric for Embankment Stabilization" measured along the surface of the ground which has been acceptably placed. No separate measurement will be made of overlapping fabric.

BASIS OF PAYMENT:

The quantity of fabric, measured as provided above, will be paid for at the contract unit price per square yard for "Fabric for Embankment Stabilization". Such price and payment will be full compensation for furnishing, hauling, placing, compaction, and all incidentals necessary to complete the work.

Pay Item: Fabric for Embankment Stabilization......Square Yard

LIGHTWEIGHT AGGREGATE

(SPECIAL)

GENERAL:

Furnish and place lightweight aggregate as shown on the plans, according to this provision, and as directed by the Engineer. Use ESCS (Expanded Shale Clay Slate) produced by the rotary kiln method conforming to ASTM C-330 (latest edition).

MATERIAL PROPERTIES:

Lightweight aggregate must have a proven record of durability, and be non-corrosive, with the following properties:

- Contains a maximum organic content of 0.1%
- Soundness Loss (AASHTO T104): Have a maximum soundness loss of less than 30% when subjected to four cycles of Magnesium Sulfate.
- Abrasion Resistance (ASTM C131): Have a maximum percentage of abrasion loss of less than 40%.
- Chloride Content (AASHTO T291): Have a chloride content of less than 100 ppm.
- Gradation (ASTM 136): use an aggregate gradation from ³/₄" to #4. Other gradations may be acceptable with approval of the Engineer.
- Aggregate loose unit weight (ASTM C29): Have a loose unit weight less than 55 lbs/ft³.
- In-place unit weight (ASTM D4253, D4254): Have an in-place compacted dry unit weight between 55 and 60 lbs/ft³. Material must be compacted to a minimum 65% relative density as determined by ASTM D4253 and D4254. Use a vibratory table when determining the maximum index density and unit weight in accordance with ASTM D4253. Determine the minimum index density and unit weight in accordance with ASTM D4254.
- Angle of Internal Friction (ASTM D3080): Minimum angle of internal friction of 40 degrees. Test a saturated representative sample (with particles larger than 0.75 inch removed) in a round or square shear box that is a minimum of 12 inches across.
- Resistivity (ASTM D1125): Have a resistivity greater than 3000 ohm-cm.
- pH (ASTM D1293): Have a pH greater than five but less than 10.

Before placing any backfill, furnish a Type IV certification in accordance with Article 106-3 of the Standard Specifications. Include a copy of all test results conducted in accordance with the above requirements in the certification. The Engineer determines how often NCDOT samples backfill material to assure compliance with gradation and other material properties.

METHOD OF CONSTRUCTION:

Place lightweight fill in uniform layers. Compact as needed to achieve the required density. Place layers not more than 12 inches in depth loose thickness and compact. Compact with three passes of an 8 – 10 ton vibratory roller in the vibratory mode, or as directed by the Engineer. In confined areas use vibratory plate compaction equipment (5 hp to 20 hp) with a minimum of two passes in 6" lifts for a 5 hp plate and 12" lifts for a 20 hp plate. Take all necessary precautions when working adjacent to the lightweight fill to ensure that the material is not over compacted. Construction equipment, other than for placement and compaction, must not operate on the exposed lightweight fill.

METHOD OF MEASUREMENT AND PAYMENT:

Lightweight aggregate will be measured and paid for per ton of "Lightweight Aggregate" that has been acceptably placed and compacted. Such price and payment will be full compensation for furnishing, hauling, placing, and compacting the fill and all incidentals necessary to complete the work.

| Pay Item: | Lightweight Aggregate | Ton |
|-----------|-----------------------|-----|
|-----------|-----------------------|-----|

SHOULDER AND FILL SLOPE MATERIAL:

(SPECIAL)

<u>DESCRIPTION</u>: Perform the required shoulder and slope construction for this project in accordance with the applicable requirements of Section 560 and Section 235 of the 2006 *Standard Specifications* except as follows:

Construct the top 12 inches of shoulder and fill slopes with soils capable of supporting vegetation.

Provide soil with a P. I. greater than 6 and less than 25 and with a pH ranging from 5.5 to 6.8. Remove stones and other foreign material 2 inches or larger in diameter. All soil in subject to test and acceptance or rejection by the Engineer.

Obtain material from within the project limits or approved borrow source.

<u>COMPENSATION</u>: When the Contractor elects to obtain material from an area located beneath a proposed fill sections which does not require excavation for any reason other than to generate acceptable shoulder and fill material, the work of performing the excavation will be considered incidental to the item of *Borrow Excavation* or *Shoulder Borrow*. If there is no pay item for Borrow or Shoulder Excavation in the contract, this work will be considered incidental to *Unclassified Excavation*. Stockpile the excavated material in a manner to facilitate measurement by the Engineer. Fill the void created by the excavation of the shoulder and fill slope material with suitable material. Payment for material used from the stockpile will be made at the contract unit price for *Borrow Excavation* or *Shoulder Borrow*, then the material will be paid for at the contract unit price for *Unclassified Excavation*. The material used to fill the void created by the excavation of the shoulder and fill slope material will be made at the contract unit price for *Unclassified Excavation*, *Borrow Excavation*, or *Shoulder Borrow*, depending on the source of the material.

Material generated from undercut excavation, unclassified excavation, or clearing and grubbing operations that is placed directly on the shoulders or slope areas, will not be measured separately for payment, as payment for work requiring the excavation will be considered adequate compensation for depositing and grading the material on the shoulder slopes.

When undercut excavation is performed at the direction of the Engineer and the material excavated is found to be suitable for use as shoulder and fill slope material, and there is no area on the project currently prepared to receive the material generated by the undercut operation, the Contractor may construct a stockpile for use as borrow at a later date. Payment for the material used from the stockpile will be made at the contract unit price for *Borrow Excavation* or *Shoulder Borrow*.

When shoulder material is obtained from the borrow sources or from stockpiled material, payment for the work of shoulder construction will be made at the contract unit price per cubic yard for *Borrow Excavation* or *Shoulder Borrow* in accordance with the applicable provisions of Section 230 or Section 560 of the 2006 *Standard Specifications*.

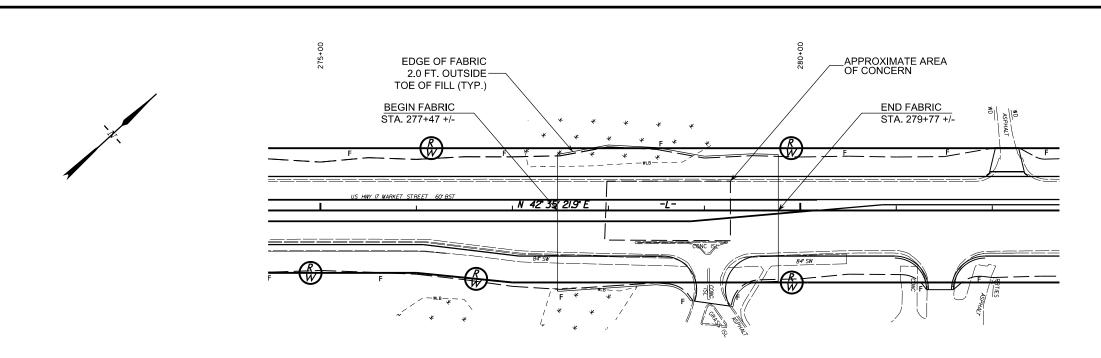
SPECIAL HANDLING OF UNCLASSIFIED EXCAVATION

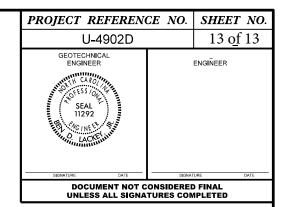
(SPECIAL)

GENERAL

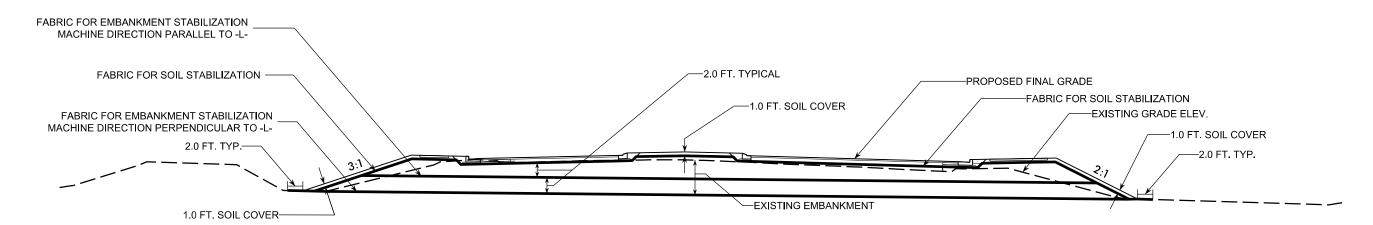
Use any suitable unclassified excavation material removed from the excavation from the stations listed below in accordance with NCDOT Standard Specification 225, except the material shall not be used in the top 3 feet of the proposed embankment or backfill. These soils may be utilized in areas outside the pavement section or in lower portions of high fills at the discretion of the Engineer.

| <u>Line</u> | Station +/- |
|-------------|------------------|
| | |
| -L- | 277+47 to 279+77 |





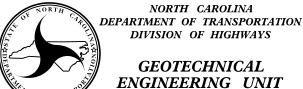
FABRIC FOR EMBANKMENT STABILIZATION LAYOUT NOT TO SCALE



REMOVE EXISTING EMBANKMENT AND REPLACE WITH LIGHTWEIGHT AGGREGATE

TYPICAL SECTION NOT TO SCALE





LIGHTWEIGHT AGGREGATE FILL EMBANKMENT

| REVISIONS | | | | | | |
|-----------|------|-----|----|------|--|--|
| BY | DATE | NO. | BY | DATE | | |
| _ | _ | 3 | 1 | _ | | |
| _ | _ | 4 | | | | |

PREPARED BY: _ Steve Hudson, P.G. DATE3/30/17

REVIEWED BY: _ Ben Lackey, P.E. DATE5/16/17



NICHOLAS J. TENNYSON Secretary

June 13, 2016

MEMORANDUM TO: David Leonard, P.E.

DDC Engineer – Division 3

DocuSigned by:

K.J. Kim

FROM: Kyung Kim, P.E.

Eastern Regional Geologian Manager

STATE PROJECT: 40238.1.4 (U-4902D)

FEDERAL PROJECT: NHS-0017(76) COUNTY: New Hanover

DESCRIPTION: US 17 Business (Market Street) from Lendire Road SR 2734

(Marsh Oaks Drive)

SUBJECT: Roadway Subsurface Inventory Report and Roadway Design

Recommendations Report

The Geotechnical Engineering Unit has reviewed and presents the following report prepared by Catlin Engineers & Scientists for the above referenced project.

Roadway Subsurface Inventory Report: 20 pages Roadway Design Recommendations Report: 5 pages

Please call Dean N Argenbright, L.G. at (252) 355-9054 or Majid Khazaei, P.E. at (919) 662-4710 if there are any questions concerning this memorandum.

Attachment KJK/DNA/MK





Telephone: (910) 452-5861 Fax: (910) 452-7563

www.catlinusa.com

June 13, 2016

MEMORANDUM TO: John L. Pilipchuk, LG, PE

State Geotechnical Engineer

FROM: Steven V. Hudson, LG

CATLIN Geotechnical Geologist

STATE PROJECT: 40238.1.4 (U-4902D)

F.A. NUMBER: NHS-0017(76) COUNTY: NEW HANOVER

DESCRIPTION: US 17 Business (Market Street) from Lendire Road to Marsh

Oaks Drive

SUBJECT: Geotechnical Report – Design and Construction

Recommendations

CATLIN Engineers and Scientists (CATLIN) has completed a subsurface investigation for this project and presents the following recommendations:

I. SLOPE/EMBANKMENT STABILITY

A. Slope Design

Recommend roadway side slopes be constructed no steeper than 3:1 (H:V) in order to assist in erosion control and establish vegetation.

B. Undercut (Soft Foundation Soils)

Include 200 cubic yards in the contract as a contingency item to be used at the discretion of the Engineer.

C. Geotextile for Soil Stabilization

Include 200 square yards of fabric for soil stabilization in the contract as a contingency item to be used at the discretion of the Engineer.

II. SUBGRADE STABILITY

A. Subsurface Drainage – Subsurface Drains

Recommend including 200 linear feet of subdrain pipe (Roadway Standard Drawing 815.02) in the contract as a contingency item to be used at the discretion of the Engineer.

B. Undercut for Subgrade Stability

Include 200 cubic yards in the contract as a contingency item to be used at the discretion of the Engineer.

C. Aggregate Subgrade

Include 100 cubic yards of shallow undercut for aggregate subgrade in the contract as a contingency item to be used at the discretion of the Engineer.

D. Geotextile for Soil Stabilization

Recommend contingency of 200 square yards of soil stabilization fabric to be used for subgrade stability as outlined in Section II.B.

Recommend 300 square yards be included in the contract for soil stabilization as a contingency item for use with aggregate subgrade as outlined in Section II.C.

Recommend an additional 200 square yards for soil stabilization be included in the contract as a contingency item to be used at the discretion of the Engineer.

III. BORROW SPECIFICATIONS

A. Borrow Criteria

Common borrow for embankment construction to subgrade shall meet Coastal Plain specifications outlined in the Standard Specifications, Article 1018-2, Section II.

B. Select Granular Material

Recommend 400 cubic yards of Select Granular Material be included as a contingency in the contract for backfill as detailed in Sections I.B and II.B.

Recommend 200 cubic yards of Select Granular Material, Class II and/or III to be included in the contract as a contingency item. Select granular material for embankment/backfill for geotextile for soil stabilization if required, or backfill in

water shall meet the criteria outlined in the Standard Specifications, Article 1016-3, Class II and/or III.

C. Shrinkage Factor

A shrinkage factor of 25 percent is recommended for calculation of earthwork in the contract.

D. Borrow Reconnaissance and Availability

Sandy soils with good to excellent engineering properties are available in nearby areas.

E. Class IV Subgrade Stabilization Material

A quantity of 190 tons of Class IV subgrade stabilization material should be included in the project contract as backfill for the Aggregate Subgrade referenced in Section II.C. The material should meet the requirement of Standard Specifications, Article 10-16-3 Class IV.

IV. MISCELLANEOUS

A. Reduction of Unclassified Excavation – Loss Due to Clearing and Grubbing

No significant loss of unclassified excavation is anticipated due to clearing and grubbing.

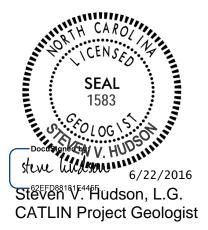
B. Reduction of Unclassified Excavation - Unsuitable Waste

Unclassified excavation will be predominantly derived from ditch and subgrade excavation and is comprised primarily of granular material which is estimated to be 100 percent suitable for embankment construction.

C. Water Wells

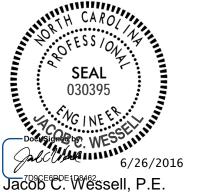
No water wells were identified with the proposed right of way limits on this project. However, if any water wells are identified during construction activities, the well/s should be sealed by a North Carolina Certified Well Contractor in accordance with the North Carolina Department of Transportation Standard Specification, Section 205, "Sealing Abandoned Wells".

Prepared By:



U4902D_GEO_Rdwy_Recs_FINAL.docx

Prepared By:



CATLIN Geotechnical Engineer

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GEOTECHNICAL ENGINEERING UNIT Summary of Quantities



- Summary of Quantities

WBS Number: 40238.1.4 County: New Hanover

TIP Number: U-4902D Field Office: GFO

Description: US 17 Business (Market Street) from Lendire Road to Marsh Oaks Drive

| Project Engineer: | JCW (CATLIN) |
|--------------------|--------------|
| Project Geologist: | SVH (CATLIN) |

| No. Quantity Adjustment Special Provision (SP) Reference Section Anginnent Station Quantity % 0036000000-E Undercut Excavation 225 - Roadway Excavation I. B Contingency N/A N/A 200 CY 003600000-E Undercut Excavation 225 - Roadway Excavation II. B Contingency N/A N/A 200 CY 019500000-E Select Granular Material 265 - Select Granular Material III. B Contingency N/A N/A 600 CY 019500000-E Select Granular Material 265 - Select Granular Material III. B Contingency N/A N/A 600 CY 019600000-E Geotextile for Soil Stabilization 270 - Geotextile for Soil Stabilization I. C Contingency N/A N/A 200 SY 109950000-E Geotextile for Soil Stabilization 270 - Geotextile for Soil Stabilization II. C Contingency N/A N/A N/A SY 109950000-E Shallow Undercut 505 - Aggregate Subgrade III. C | Pay Item | Pay Item/ | Spec Book Section No. or | Report | Alignment | Begin | End | Quantity | Units / |
|---|---|-----------------------------------|---|----------|-----------------|---------------|---------------|----------|---------|
| 0036000000-E Undercut Excavation 225 - Roadway Excavation II. B Contingency N/A N/A 200 CY 0195000000-E Select Granular Material 265 - Select Granular Material III. B Contingency N/A N/A 600 CY 0196000000-E Geotextile for Soil Stabilization 270 - Geotextile for Soil Stabilization I. C Contingency N/A N/A 800 CY 0196000000-E Geotextile for Soil Stabilization 270 - Geotextile for Soil Stabilization I. C Contingency N/A N/A N/A 200 SY 1099500000-E Geotextile for Soil Stabilization 270 - Geotextile for Soil Stabilization II. D Contingency N/A N/A N/A 700 SY 1099500000-E Shallow Undercut 505 - Aggregate Subgrade II. C Contingency N/A N/A N/A 100 CY 1099700000-E Shallow Undercut 505 - Aggregate Subgrade III. E Contingency N/A N/A N/A TON 1099700000-E | No. | Quantity Adjustment | Special Provision (SP) Reference | Section | Angillient | Station | Station | Quantity | % |
| Total Quantity of Undercut Excavation = 400 CY 0195000000-E Select Granular Material 265 - Select Granular Material III. B Contingency N/A N/A 600 CY 0196000000-E Geotextile for Soil Stabilization 270 - Geotextile for Soil Stabilization II. C Contingency N/A N/A 200 SY 0196000000-E Geotextile for Soil Stabilization 270 - Geotextile for Soil Stabilization II. D Contingency N/A N/A 700 SY 1099500000-E Shallow Undercut 505 - Aggregate Subgrade II. C Contingency N/A N/A 100 CY 1099700000-E Class IV Subgrade Stabilization 505 - Aggregate Subgrade III. E Contingency N/A N/A 190 TON 1099700000-E Class IV Subgrade Stabilization 505 - Aggregate Subgrade III. E Contingency N/A N/A 190 TON 2044000000-E 6" Perforated Subdrain Pipe 815 - Subsurface Drainage II. A Contingency N/A N/A N/A 200 LF | 0036000000-Е | Undercut Excavation | 225 - Roadway Excavation | I. B | Contingency | N/A | N/A | 200 | CY |
| Oly5000000-E Select Granular Material 265 - Select Granular Material III. B Contingency N/A N/A 600 CY | 0036000000-Е | Undercut Excavation | 225 - Roadway Excavation | II. B | Contingency | N/A | N/A | 200 | CY |
| Total Quantity of Select Granular Material = 600 CY 0196000000-E Geotextile for Soil Stabilization 270 - Geotextile for Soil Stabilization II. C Contingency N/A N/A 200 SY 0196000000-E Geotextile for Soil Stabilization 270 - Geotextile for Soil Stabilization II. D Contingency N/A N/A 700 SY Total Quantity of Geotextile for Soil Stabilization 900 SY 1099500000-E Shallow Undercut 505 - Aggregate Subgrade II. C Contingency N/A N/A 100 CY 1099700000-E Class IV Subgrade Stabilization 505 - Aggregate Subgrade III. E Contingency N/A N/A 190 TON Total Quantity of Class IV Subgrade Stabilization 190 TON 2044000000-E 6" Perforated Subdrain Pipe 815 - Subsurface Drainage II. A Contingency N/A N/A N/A 200 LF | | | | T | otal Quantity | of Undercut | Excavation = | 400 | CY |
| 0196000000-EGeotextile for Soil Stabilization270 - Geotextile for Soil StabilizationI. CContingencyN/AN/A200SY019600000-EGeotextile for Soil Stabilization270 - Geotextile for Soil StabilizationII. DContingencyN/AN/AN/A700SYTotal Quantity of Geotextile for Soil Stabilization900SY1099500000-EShallow Undercut505 - Aggregate SubgradeIII. CContingencyN/AN/A100CY1099700000-EClass IV Subgrade Stabilization505 - Aggregate SubgradeIII. EContingencyN/AN/A190TON2044000000-E6" Perforated Subdrain Pipe815 - Subsurface DrainageII. AContingencyN/AN/AN/A200LF | 0195000000-E | Select Granular Material | 265 - Select Granular Material | III. B | Contingency | N/A | N/A | 600 | CY |
| 019600000-EGeotextile for Soil Stabilization270 - Geotextile for Soil StabilizationII. DContingencyN/AN/A700SYTotal Quantity of Geotextile for Soil Stabilization = 900SY1099500000-EShallow Undercut505 - Aggregate SubgradeII. CContingencyN/AN/A100CY109970000-EClass IV Subgrade Stabilization505 - Aggregate SubgradeIII. EContingencyN/AN/A190TON2044000000-E6" Perforated Subdrain Pipe815 - Subsurface DrainageII. AContingencyN/AN/AN/A200LF | , | | | Total | Quantity of S | elect Granula | ar Material = | 600 | CY |
| Total Quantity of Geotextile for Soil Stabilization = 900 SY 1099500000-E Shallow Undercut 505 - Aggregate Subgrade II. C Contingency N/A N/A 100 CY 1099700000-E Class IV Subgrade Stabilization 505 - Aggregate Subgrade III. E Contingency N/A N/A 190 TON 1099700000-E Class IV Subgrade Stabilization 505 - Aggregate Subgrade III. E Contingency N/A N/A 190 TON 1099700000-E 6" Perforated Subdrain Pipe 815 - Subsurface Drainage II. A Contingency N/A N/A 200 LF | 0196000000-E | Geotextile for Soil Stabilization | 270 - Geotextile for Soil Stabilization | I. C | Contingency | N/A | N/A | 200 | SY |
| 109950000-E Shallow Undercut 505 - Aggregate Subgrade II. C Contingency N/A N/A 100 CY Total Quantity of Shallow Undercut = 100 CY 109970000-E Class IV Subgrade Stabilization 505 - Aggregate Subgrade III. E Contingency N/A N/A 190 TON Total Quantity of Class IV Subgrade Stabilization = 190 TON 204400000-E 6" Perforated Subdrain Pipe 815 - Subsurface Drainage II. A Contingency N/A N/A 200 LF | 0196000000-E | Geotextile for Soil Stabilization | 270 - Geotextile for Soil Stabilization | II. D | Contingency | N/A | N/A | 700 | SY |
| Total Quantity of Shallow Undercut = 100 CY 1099700000-E Class IV Subgrade Stabilization 505 - Aggregate Subgrade III. E Contingency N/A N/A 190 TON Total Quantity of Class IV Subgrade Stabilization = 190 TON 2044000000-E 6" Perforated Subdrain Pipe 815 - Subsurface Drainage II. A Contingency N/A N/A 200 LF | Total Quantity of Geotextile for Soil Stabilization = | | | | | 900 | SY | | |
| 109970000-E Class IV Subgrade Stabilization 505 - Aggregate Subgrade III. E Contingency N/A N/A 190 TON Total Quantity of Class IV Subgrade Stabilization = 190 TON 204400000-E 6" Perforated Subdrain Pipe 815 - Subsurface Drainage II. A Contingency N/A N/A 200 LF | 1099500000-E | Shallow Undercut | 505 - Aggregate Subgrade | II. C | Contingency | N/A | N/A | 100 | CY |
| Total Quantity of Class IV Subgrade Stabilization = 190 TON 204400000-E 6" Perforated Subdrain Pipe 815 - Subsurface Drainage II. A Contingency N/A N/A 200 LF | | | | | | | 100 | CY | |
| 2044000000-E 6" Perforated Subdrain Pipe 815 - Subsurface Drainage II. A Contingency N/A N/A 200 LF | 1099700000-E | Class IV Subgrade Stabilization | 505 - Aggregate Subgrade | III. E | Contingency | N/A | N/A | 190 | TON |
| | Total Quantity of Class IV Subgrade Stabilization = | | | | | 190 | TON | | |
| Total Quantity of 6" Perforated Subdrain Pipe = 200 LF | 2044000000-Е | 6" Perforated Subdrain Pipe | 815 - Subsurface Drainage | II. A | Contingency | N/A | N/A | 200 | LF |
| | | | | Total Qu | antity of 6" Po | erforated Sub | drain Pipe = | 200 | LF |

| These Items Only Impact Earthwork Totals | | | | | | | | |
|--|------------------|-------------------|--------|-----|-----|-----|----|---|
| N/A | Shrinkage Factor | 235 - Embankments | III. C | N/A | N/A | N/A | 25 | % |